



1. Analysis - Using Gross Section Properties

PRESTRESSED CONCRETE SECTION PROPERTIES

(VERSION 2.00)

PAGE 2

INPUT DATA

	P															
	R															
	S															
T	T															
R	R															
S	S															
A, T	STRENGTH GRADE: P-5 SPAN == COMPOSITE SLAB -- T-S	X	X	L	FINAL	LIVE	SO	TO THE SO	TO							
A, R	DEPTH DEPTH SPAC: P-5 LONG THICKNESS DEPR: P-5 S-N P-5	C	S	A ALL	PREST	LOAD	INITIAL	INTL. FINAL	FINAL							
	(100) (170) (100) (100) (100) (100) (100) (100) (100) (100)															
1.0	50.0 54.0 64 4.0 80.	7.1	84.	.6	3.3	1.1	272	4.0	24.0	T 1.0	0.	.00	0	0	200	0

RESULTS TRIAL NO. 1 PRE-TENSIONED

GROSS SECTION PROPERTIES - GROSS: PROPERTIES CALCULATED FROM BUILT-UP SECTION:

AREA	138	AF	AF2	I300	I300	
(IN ²)	(100)	(100)	(100)	(100)	(100)	
WEIGHT PER SEC	558.0	25.7	14364.	464175.	92241.	186658.

GROSS COMPOSITE SECTION PROPERTIES: (PROPERTIES CALCULATED FROM BUILT-UP SECTION):

AREA	138	AF	AF2	I300	I300	
(IN ²)	(100)	(100)	(100)	(100)	(100)	
SLAB ONLY	358.0	25.7	14364.	464175.	92241.	186658.
SLAB ONLY	358.4	25.4	34860.	203754.	2505.	
FILLET ONLY	17.1	54.4	931.	30688.	1.	
ONRIG ONLY	0	0	0	0	0	
COMP SEC W/P	1171.5	42.0	56155.	2852415.	94749.	439923.

CALCULATED SIMPLY SUPPORTED BEAM MOMENTS

M _{D1} = DEAD LOAD OF SLAB ONLY	= 405.1	
M _{D2} = DEAD LOAD OF SLAB + FILLETS	= 511.4	
M _{D3} = ADDED DEAD LOAD	= 250.0	INPUTTED VALUE
M _{D4} = LIVE LOAD MOMENT	= 922.1	1000-44 TRUCK LOADING

M_{MAX} = ULTIMATE MOMENT APPLIED = 3801.7 1.2*(D2+D3+D4) + 1.3*(P11 TRUCK)

PRESTRESSED CONCRETE ANALYSIS

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STRENGTH AND FRESHNESS - USING GROSS CROSS-SECTIONS

MOMENT (K')	FT (PSI)	PS (PSI)	
SL. CONCRETE	465.	945.	-776.
SL. SLAB + FILLET	511.	929.	-846.
ADDED DL	250.	54.	-206.
TOTAL DL	1171.	1828.	-1822.
(LMAX)E	922.	248.	-949.
TOTAL DL + (LMAX)E	---	2775.	-2769.

DESIGN STRESS = P DS (PSI) GENERATOR OF THE FOLLOWING:

PS = 1 * STRENGTH C' = ALL. TEMP. FACTOR = 1.00.	PS = C' * ADD. DL	PS = 1.00.
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The gross section properties are calculated by building up parts determined by the section dimensions. AY, AYZ and I_{ZZ} are the summations of the properties of the parts used to build up the section, these numbers can vary depending on how the section is split into parts. The area, YB, and ITOT are the overall section properties.

- The calculated moments are the program calculated simple beam moments or the moments input by the user.
 - The ultimate applied moment is calculated using the beam moments listed. If the live load moment is calculated by the program, then the



P-truck moment will also be calculated to determine the maximum applied moment.

- Note the ultimate applied moment = $0.95 \times$ nominal moment strength of a section.

2. Analysis - Using Transformed Section Properties

FINAL TRANSFORMED GIRDERS PROPERTIES		(PROPERTIES CALCULATED FROM SPILT-UP SECTIONS)			
		$I_{TOT} = 100 + A_{ST} - S_{CN} (A_Y * Y_B)$			
	AREA	YB	A _Y	A _{Y2}	I _{TOT}
	(IN ²)	(IN)	(IN ²)	(IN ²)	(IN ⁴)
GROSS CCR SEC	556.0	25.7	14364.	464175.	92241.
(H-1)*AS ²	17.8	4.0	71.	283.	---
TRAN CCR SEC	575.8	25.1	14435.	464460.	92241.
					194408.
FINAL TRANSFORMED COMPOSITE GIRDERS PROPERTIES		(PROPERTIES CALCULATED FROM SPILT-UP SECTIONS)			
		$I_{TOT} = 100 + A_{ST} - S_{CN} (A_Y * Y_B)$			
	AREA	YB	A _Y	A _{Y2}	I _{TOT}
	(IN ²)	(IN)	(IN ²)	(IN ²)	(IN ⁴)
TRAN COMP GCR	1189.3	47.2	50221.	2552699.	94748.
(E ² *I _B)/R ² *2 =	1.8613				524315.
STRESSES W/O PRETENSION - USING FINAL TRANSFORMED CROSS-SECTIONS					
	MOMENT (IN ³)	PF (PSI)	FB (PSI)		
DL GIRDER	468.	829.	-318.		
DL STAR + PELLET	511.	911.	-780.		
ADDED DL	200.	54.	-193.		
TOTAL DL	1177.	1754.	-1701.		
(DL+I)E	921.	247.	-881.		
TOTAL DL + (DL+I)	---	2041.	-2581.		
DESIGN STRESS = F DCR (PSI) (GREATER OF THE FOLLOWING)					
PF = E * SQRT(I _B /C) * ALL. TEN. FACTOR =	2239.	PF (DC + ADD. DL)	=	1701.	
PRESTRESSED GIRDER ANALYSIS PAGE 3					
PRESTRESSING FORCE = PRETENSIONED					
ALLOW. TENSION = (F _T * .75) = 167500. (PSI)		FINAL PRESTRESS FORCE = PF = 496591. (LBS)			
AREA OF PRESTRESSING STEEL = A _{PT} = 2.96 (IN ²)		INITIAL PRESTRESS FORCE = PI = 541815. (LBS)			
STRESSES WITH PRETENSION - USING FINAL TRANSFORMED CROSS-SECTIONS (PSI)					
AT MIDSPAN	TOP FIBER	BOTTOM FIBER			
# STRENGTH (PSI)	47.	(PSI)	1781.		
# DL GIRDER+STAR (PSI)	1349.	(PSI)	701.		
# FULL DES. LOAD (PSI)	1350.	(PSI)	-180.		
AT 1/3 POINT ALONG SPAN LENGTH	LOADING HANDED AT 1/3 POINT				
# STRENGTH (PSI)	-44.	(PSI)	1840.		
# FULL DEAD LOAD (PSI)	857.	(PSI)	867.		
# FULL DES. LOAD (PSI)	1128.	(PSI)	-93.		
CONCRETE STRENGTH REQUIRED (PSI)					
PCI = FMAX/.40 = 3198.	PC = FMAX/.40 = 3371.	FMAX (PSI)	=	4000.	
DEFLECTIONS (INCHES) (DOWNWARD POSITIVE)					
DIM = -1.6687	DOH = .7630	DIW = -.9017	DPW = -1.0932	DSLW = .8369	DAW = .1214
					DPW = -.3229
CHECK ULTIMATE RESISTING MOMENT					
RCMF. INDEX = .043		S.U. RESIST. MOMENT (K-FT) = 3439.62			
RCU (PSI) = 263219.		S.A. @ ULTIMATE LOAD (IN) = 3.90			

- Transformed section properties are calculated using $N = 7$. The net transformed girder properties are calculated for post-tensioned girders.
- Note: Stresses with prestress are calculated at 1/3 span only for pretensioned girders with program calculated live load moment.
- The calculated deflection are based on the input P/S f_c or P/S f_c max required.
- The distance between top of the slab and the natural axis at the ultimate load is calculated for the girder section. However, the value may not be correct for some non-standard sections. For example, trapezoidal sections.

3. Results - Final Prestressing Force Calculated

A sample output is shown below:

PRESTRESSED GIRDER ANALYSIS												PAGE 4				
RESULTS - FINAL PRESTRESSING FORCE CALCULATED																
TOTAL NO.	STRESSES		CONCRETE STRENGTH	FINAL FIBER	ELEMENT MOMENT	DEFLECTIONS				SECTION PROPERTIES						
	INITIAL	FINAL				(INCH MM)	(INCH MM)	(INCH MM)	(INCH MM)	GIRDER	COMPOSITE	I	A	T _b	I	A
TOP BOTTOM	TOP BOTTOM	UNIT. FINAL FORCE	APPLIED MOMENT	SPAN	DEFL	DEFL	DEFL	DEFL	MATERIAL	SECTION	SECTION	(IN)	(MM)	(IN)	(IN)	(MM)
(KSI)	(KSI)	(KSI)	(KSI)	(KSI)	(IN)	(MM)	(IN)	(MM)	(INCH)	(INCH)	(INCH)	(IN)	(MM)	(IN)	(MM)	(IN)
0 1	47.1761	1200. -380.	3.10	3.37	497.	3802.7	3479.6	-1.39	+.375	.070	-.010	1948.7	276	25.1	5261.8	42.2
NOTE: DEFLECTIONS ARE POSITIVE DOWNWARD																
NOTE: DEFLECTIONS ARE BASED ON INITIAL P-S F/C																
NOTE: FOR P-S CONCRETE F/C USE 4000, F/C																
NOTE: APPLIED ULTIMATE MOMENT EXCEEDS ULTIMATE RESISTING MOMENT. REINFORCING STEEL IS REQUIRED.																

This report contains the following output:

- Midspan Fiber Stresses, both Initial (at the time of stressing) and Final (under full design load) in psi (English Units) or kPa (Metric Units).
- Concrete Strength Requirement, both Initial (temporary stress condition) and Final (design load stress condition) in psi (English Units) or kPa (Metric Units).
- Final prestressing force required in kips (English Units) or kN (Metric Units).



- Ultimate Moments, both applied and resisting, in kip-ft (English Units) or kN-m (Metric Units). If a moment is entered by the user under Note D, that moment is used to calculate the ultimate applied moment rather than any program calculated live load moment for HS and P ultimate moment.
- Deflections in feet - four deflections (DPI, DEF1, DEF2, DEF3). These deflections have different meanings when analyzing the structure as a box girder than they do when analyzing the structure as single stringer elements. Deflections are positive downward. To interpret your results, see the chart below:

Deflection No.	Cause of Deflection	
	Box Girder (Non-Composite)	Single Girder (With Composite Slab)
DPI		Stressing Only
DEF1	Stressing Only	Stressing + DL Girder
DEF2	DL Only	DL Slab
DEF3	DL Additional	All Above + Creep

For a single Girder, DEF3 is the total amount the girder deflects from its original position. The deflection due to creep is not listed but may be found by adding DEF2 and DEF3 and subtracting DEF1 from the sum. A coefficient of creep of 1.5 is used by the program. See Appendix 1 for deflection evaluations.

Section Properties - Results shown under Girder are the moment of Inertia (I), the area (A) and the distance from the bottom of the girder or soffit to the centroidal axial of the cross section (YB). For composite girder construction the results shown are for the girder only. Results shown under composites are the moment of inertia (I), and the distance from the bottom of the girder or soffit to the the centroidal axial of the cross section (YB). For composite girder construction, the top slab has been included. For a noncomposite construction, the I and YB should be same as shown under Girder. Moments of inertias are in in⁴ or mm⁴, areas are in in² or mm², and distances are in inches or millimeters.

Note: For the output in metric units, "10⁴" is shown as "10^4" and "10⁶" is shown as "10^6".



4. Results - Final Prestressing Force Given

A sample output is shown below:

PRESTRESSED GIRDER ANALYSIS												PAGE 4	
RESULTS - FINAL PRESTRESSING FORCE GIVEN													
TOTAL INPUT NO. FIBER	CONCRETE STRENGTH		ULTIMATE MOMENT			DEFLECTIONS (IN INCHES)			FIBER STRESSES		SECTION PROPERTIES		
	INIT.	FINAL	APPLIED	RESID.	SPAN	SPAN1	SPAN2	SPAN3	TOP	BOT.	A	I	YB
0 1 457. 3.10 3.37 3801.7 3482.3 -.129 -.076 .070 -.016 1785. 571. 194814. 25.1									PLATE SLAB	1048. 703. 574. 134814. 25.1			
									ADD. D.L.	1102. 810. 1189. 126340. 42.2			
									FULL DES. LOAD	1349. -378. 1189. 126340. 42.2			

NOTE: DEFLECTIONS ARE POSITIVE DOWNWARD
NOTE: DEFLECTIONS ARE BASED ON INPUT 3-6 7°C
NOTE: FOR P-S CONCRETE F' C 4000, PSF
NOTE: APPLIED ULTIMATE MOMENT EXCEEDS ULTIMATE RESISTING MOMENT. HELD STRESS IS REQUIRED.

This report contains the following output:

- Concrete Strength Requirement both initial and final in ksi (English Units) or MPa (Metric Units).
- Ultimate Moments both applied and resisting in kip-ft (English Units) or kN-m (Metric Units).
- Deflections in feet (English Units) or m (Metric Units).
- Midspan fiber stresses at the time of stressing, after slab placement, after addition of added dead load, and under full design load. Results are in psi (English Units) or kPa (Metric Units).
- Section Properties: Results shown are the moments of inertia (I) in in⁴ (English Units) or mm⁴ (Metric Units), the area (A) in in² (English Units) or mm² (Metric Units), and the distance (YB) in inches (English Units) or mm (Metric Units) from the bottom of the girder or soffit to the cross section.
- Notes are included for the P-S concrete f'_c to use (either minimum allowed by the specs or maximum required by the analysis), if the applied ultimate moment exceeds the ultimate resisting moment, or if the allowable tension has been exceeded.



The following table shows the section properties and the elements considered for the various stages of a pretensioned bridge:

Loading Stage	Method of Calculation	Elements Considered	Remarks
Time of Stress	Transformed Sec./Net	Girder Only	
Slab Placement	Transformed Sec./Gross	Girder Only	
Added D.L.	Transformed Sec./Gross	Girder and Slab	Composite Girder
Added D.L.	Transformed Sec./Gross	Girder Only	Non-Composite Girder
Full Design Load			Same as added D.L.

The following table shows the section properties and the elements considered for the various stages of a pretensioned bridge:

Loading Stage	Method of Calculation	Elements Considered	Remarks
Time of Stress	Transformed Sec./Net	Total Cross Section	
Slab Placement			Ignore Results
Added D.L.	Transformed Sec./Gross	Total Cross Section Excluding Rails	
Full Design Load			Same as added D.L.

For more information on the results calculated for ultimate moments and deflections, see the "Results - Final Prestressing Force Calculated" portion of these instructions.



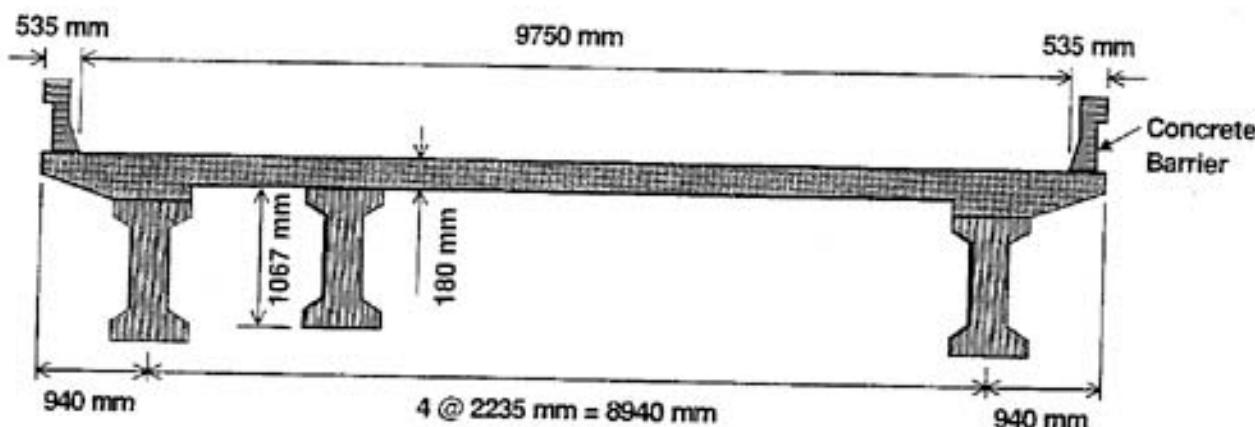
Example Problems

Attached are example problems to illustrate how the program works. Only important portions of the example problems are shown here. The following table lists the type of bridge being analyzed, and the special characteristics of each of the sample problems.

Problem No.	Bridge Type	Units Used	Special Characteristics
1	Std. "I" Girder	Metric	None.
2	Std. "I" Girder	English	Continuous for LL using max HS20 LL + IM.
3	Std. "I" Girder	Metric	Illustrates use of Symmetric Section Dimensions Sheet.
4 (with Text file)	Non-Std. "I" Girder	Metric	Illustrates use of Non-Symmetric Section Dimensions Sheet.
5	Special Girder	English	Illustrates use of Section by Parts Sheet. All moments are specified.
6 (with Text file)	Cored Slab	English	Illustrates Analysis of Cored Slab Bridge.

Example Problem 1 (Metric)

Simply Supported



Design span = 20.7 m
 DL moment barrier + AC = 298 kN-m

Table No. Section No.	G-dr. Properties										Prestress Data						Moments			
	Concrete Sect.										Prestress Data						Moments			
	Girder Depth	Girder Depth	Girder Depth	Girder Spacing	Fs	Fr	Span Length	Thickness	Width	Super	Fs	Fr	Total Prestress Force	Prestressing Beam	Prestressing Beam	Final Prestress Force	Live Load Lanes	Note A	Note B	Note C
	mm	mm	mm	mm (ft)			mm	mm	mm	mm	mm	mm	kN	mm	mm	mm	mm	mm	mm	mm
1	11200	41067	2035	1.4	20.7	118.0	2035	12	12	12	12	12	100	100	100	1.4	1.4	1.4	1.4	1.4
2																				
3																				



Output for Example Problem 1:

CALIFORNIA DEPARTMENT OF TRANSPORTATION									
DIVISION OF STRUCTURES - DESIGNS									
PRESTRESSED GIRDER ANALYSIS									
(VERSION 2.00)									
INPUT DATA									
T S	R E	Z C	A T	L W	X Y Z	X Y Z	SLW	SLW	SLW
STEEL GIRDERS	SPAN -- COMPOSITE SLAB	Y' C	Y' C	DEPTH SPAC	Y' C	Y' C	APPLIED	APPLIED	APPLIED
1.0 1273 1667 2235 26.0 26.7	181 2235	.0 22.5	1.1 1462	102	304 Y 1.0	0.	.30	0	0
(mm)	(mm)	(m)	(mm)	(mm)	(mm)	(mm)	(kN-m)	(kN-m)	(kN-m)
NOTE: ARMED DATA IS BRACKETED BY COLONS.									
ANALYSIS	TRAIL NO. 1	PRE-TENSIONED							
0	GROSS SECTION PROPERTIES - GIRDERS	(PROPERTIES CALCULATED FROM BUILT-UP SECTION)							
	AREA	YR	AY	AYZ	I00	I00	I00	I00	I00
	(10 ³ mm ²)	(mm)	(10 ³ mm ³)	(10 ⁶ mm ⁴)	(10 ⁶ mm ⁴)	(10 ⁶ mm ⁴)	(10 ⁶ mm ⁴)		
0	GROSS GIRD. SEC.	304	507	155200	169315	18160	39738		
0	GROSS COMPOSITE SECTION PROPERTIES	(PROPERTIES CALCULATED FROM BUILT-UP SECTION)							
	AREA	YR	AY	AYZ	I00	I00	I00	I00	I00
	GIRD. ONLY	304	507	155200	169315	18160	39738		
	SLAB ONLY	402	1143	475921	583014	10465			
	FILLET ONLY	13	1086	133551	14426	1			
	OVERH. ONLY	0	0	0	0	0			
	COMP. SEC. PROP.	721	895	644572	877945	11047	120558		
CALCULATED SIMPLY SUPPORTED BEAM MOMENTS									
DML = DEAD LOAD OF GIRDER ONLY	=	346.2							
DMG = DEAD LOAD OF SLAB + FILLETS	=	523.3							
ADL = ADDED DEAD LOAD	=	298.3	INPUTTED VALUE						
LLW = LIVE LOAD MOMENT	=	1079.4	R320-44 TRUCK LOADING						
MUMA = ULTIMATE MOMENT APPLIED	=	3922.4	1.3*(DML+DMG+ADL) + 1.3*(LLW TRUCK)						
STRESSES W/O ENTHARGING - USING GROSS CROSS-SECTIONS									
SL. GIRDER	MOMENT (kN-m)	YR (kPa)	YR (kPa)						
	346.2	5437.	-4931.						
SL. SLAB + FILLET	524.1	7375.	-6619.						
ADDED SL.	298.3	427.	-2213.						
TOTAL SL.	1268.	13229.	-13833.						
(LL+TIE)	1079.	1564.	-6006.						
TOTAL SL. + (LL+TIE)	---	14793.	-21841.						
DESIGN STRENGTH - F DRS (kPa) (GREATER OF THE FOLLOWING)									
FS = C.5*SQRT(F'c') * ALL. TIE. FACTOR	=	19200.	YR (SL + ADD. SL)						
NOTE :- REAL CALCULATIONS ARE IN ENGLISH UNITS									

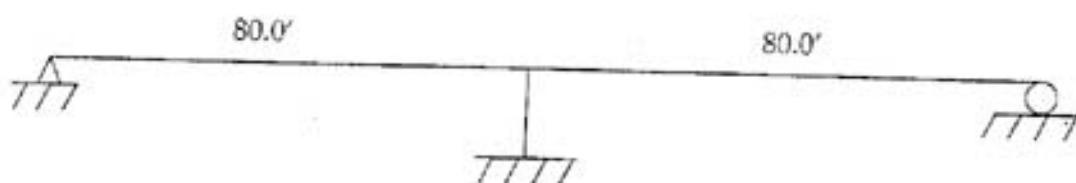


C FINAL TRANSFORMED COMPOSITE PROPERTIES		(PROPERTIES CALCULATED FROM BUILT-UP SECTION)															
			($\Sigma M = 100 + A_{st} - S_{st} (A_f * 10)$)														
	A _f	10	A _{f1}	A _{f2}	100	S _{st}											
	(10^3 mm^2)	(mm)	(10^3 mm^2)	(10^3 mm^2)	(10^3 mm^2)	(10^3 mm^2)											
GROSS CON SEC	308	507	155200	100330	18160	39736											
IN-17 TAS ¹	12	103	1200	120	—	—											
TRANS CON SEC	318	492	156402	100458	18160	41604											
D FINAL TRANSFORMED COMPOSITE CONCRETE PROPERTIES		(PROPERTIES CALCULATED FROM BUILT-UP SECTION)															
			($\Sigma M = 100 + A_{st} - S_{st} (A_f * 10)$)														
	A _f	10	A _{f1}	A _{f2}	100	S _{st}											
TRANS COMP CON	318	603	645174	670337	12247	127641											
	$(S_{st}) / (M_{st}) = 1.7126$		$(M_{st}) / (M_{st}) = 1.4677$														
STRENGTH AND PRESTRESS - USING FINAL TRANSFORMED CROSS-SECTIONS																	
	MOMENT (KN-m)	PT (kPa)	PR (kPa)														
SL. GIRDERS	386.	5113.	-4370.														
SL. STAB + FILLET	524.	7234.	-6200.														
ADDED SL.	298.	412.	-2057.														
TOTAL SL.	1208.	12999.	-12227.														
SL+CSL	1079.	1564.	-7444.														
TOTAL SL + (CSL-C)	—	14563.	-90271.														
DESIGN STRESS = P SL (kPa) (APPROX OF THE FOLLOWING)																	
PT = 0.85 Q _{0.6} (P/C) * ALL. TENS. FACTOR = 1762.				PT SL + ADD. SL		= 12627.											
NOTE : - REAL CALCULATIONS ARE IN ENGLISH UNITS																	
PRESTRESSING FORCE - PRESTRESSED																	
ALLOW. TENSION = ($P_t + 1.75 - 241.32$) = 1154913. (kPa)				FINAL PRESTRESS FORCE = PT = 2246231. (kN)													
AREA OF PRESTRESSING STEEL = A _f = 1.96 (10^3 mm^2) INITIAL PRESTRESS FORCE = P _t = 2041148. (kN)																	
STRENGTH WITH PRESTRESS - USING FINAL TRANSFORMED CROSS-SECTIONS (kPa)																	
	AT MIDSPAN	TOP FIBER	BOTTOM FIBER														
	# THERMING	(PT) ₁	(PT) ₂														
	# SL. GIRDERS+SL. (PT) ₁	7476.	6854.														
	# FULL LOAD (PT) ₁	9474.	8387.														
	AT 1/3 POINT ALONG SPAN LENGTH	THERMING HAPPEL AT 1/3 POSITION															
		ASSUME SL. OR CS. STRESS AT 1/3 PT. = 0.85 * STRESS + CLO															
	# THERMING	(PT) ₂	(PT) ₁														
	# FULL DEAD LOAD (PT) ₂	6255.	6539.														
	# FULL LOAD (PT) ₂	7872.	7187.														
	CONCRETE STRENGTH REQUIRED (kPa)																
	PT ₁ = PT _{MAX} / .60 = 26442.	PT ₂ = PT _{MAX} / .40 = 33667.	PT _{MAX} (PT ₂) = 27994.														
	DEFLECTIONS (mm)	(DEMAND POSITIVE)															
	DE ₁ = -43.57	DE ₂ = 16.54	DE ₃ = -27.03	DE ₄ = -31.35	DE ₅ = 22.44	DE ₆ = 4.14											
						EF = -6.76											
	CHECK ULTIMATE RESISTING MOMENT																
	PREST. INDEX = .002		SL. PREST. MOMENT (kN-m) = 3793.54														
	PT ₁ (kPa) = 1403851.		N.A. & ULTIMATE LOAD (kN) = 57.52														
	RESULTS - FINAL PRESTRESSING FORCE CALCULATED																
	TRAIL. STRESSES	CONCRETE	FINAL	ULTRAMAX	DISPLACEMENTS	SECTION PROPERTIES											
NO.	INITIAL	FINAL	STRENGTH	MOMENT	(MM MM)	CONCRETE	COMPOSITE										
	TOP BOTTOM	TOP BOTTOM	INT. FINAL FORCE	APPLIED	A _f	A _{f1}	A _{f2}	I	A	10	I	10					
	(kPa) (kPa)	(kPa) (kPa)	(kN)	(kN)	(kN-m)	(kN-m)	(kN-m)	(mm)	(mm)	(mm)	(mm)	(mm)					
0 1	-424 15368	9474	-2646	26.4	23.7	22.6	3522	3739	-43.6	-27.0	22.4	-6.8	41604	317	492	12784	602
	NOTE: DEFLECTIONS ARE POSITIVE DEMAND																
	NOTE: DEFLECTIONS ARE BASED ON DEPUTY P-0 F/C																
	NOTE: FOR P-0 CONCRETE F/C USE 27994. (kPa)																
	NOTE: APPLIED ULTIMATE MOMENT EXCEEDS ULTIMATE RESISTING MOMENT, HELD STEEL IS REQUIRED.																

Example Problem 2 (English)

Simply supported for dead load of girder.

Span continuous for added dead load and live load.



Design of Span 1

Span Length = 80 ft Girder Spacing = 8'-6" Girder Depth = 4'-6"

Top Slab = 7.625" Structure Depth = 5'-4"

- Added DL Moment = 195
- (LL+D)_H = 888 k-ft; (LL + D)_P = 1618 k-ft
- Moments or continuous spans must be determined by an analysis program that allows continuous spans (eg., Frame, BDS, etc.).
- Moments are at centerline of span.
- P-Truck moment to be input = $\frac{1618}{5/3} = 971$ k-ft

Trial No.	Section No.	Girder Properties										Prestress Data							Moments			
		Sin. Class (#)	Girder Depth (in.)	Girder Span (ft)	Girder Spacing (ft)	P.S. (in.)	Sp. Length (in.)	Thick. (in.)	Width (in.)	Super. (in.)	# (in.)	Type Connection	f _a (ksi)	XDL (in.)	XND (in.)	Low Tens. (in.)	High Tens. (in.)	Flexi. Force (kips)	Live Load Lanes	Note A DLM Applied to Initial Section	Note B DLM Applied to TFD Initial Section	Note C DLM Applied to Final Section
11	16140	16140	100-2	80.0	124.6	100-2	4.4	4.4	1.1	1.1	1.1	160	21740	1140	1140	1140	1140	1140	1	1140	1140	1140
12	16140	16140	100-2	80.0	124.6	100-2	4.4	4.4	1.1	1.1	1.1	160	21740	1140	1140	1140	1140	1140	1	1140	1140	1140
13	16140	16140	100-2	80.0	124.6	100-2	4.4	4.4	1.1	1.1	1.1	160	21740	1140	1140	1140	1140	1140	1	1140	1140	1140



Output for Example Problem 2:

CALIFORNIA DEPARTMENT OF TRANSPORTATION PERIODIC CEDER ANALYSIS (VERSION 2.00)										PAGE
INPUT DATA										
T S				T						
N S				Z						
T C				E						
A T				S						
L N										
1 D										
Y N										
Y M				X		X	L	FINAL LINE	TO	TO SPAN ID TO
Y C				C		M		A ALL PREST LOAD	INITIAL SPAN	FINAL SPAN
1 H				L		S		X DEF FORCE LAMES SECTION	SECTION SPAN	SECTION SPAN
(IN.)	(IN.)	(IN.)	(IN.)	(IN.)	(IN.)	(IN.)	(IN.)	(IN.)	(IN.)	(IN.)
1.0	61.0	34.0	303	(4.0)	80.	7.6	102.	.0	12.3	1.1
1.0	61.0	34.0	303	(4.0)	80.	7.6	102.	.0	12.3	1.1
1.0	61.0	34.0	303	(4.0)	80.	7.6	102.	.0	12.3	1.1
NOTE: ASSUMED DATA IS ENCLOSED BY COLONS.										
ANALYSIS	TRAIL NO. 1		PRE-TRANSFORMED							
GROSS SECTION PROPERTIES - CEDER	(PROPERTIES CALCULATED FROM BUILT-UP SECTION)									
	AREA	12	ZY	AZ2	120	ZY	ITOT			
	(IN2)	(IN2)	(IN2)	(IN2)	(IN2)	(IN2)	(IN2)			
GROSS GER SEC	558.0	25.7	14364.	464175.	92241.	186659.				
GROSS COMPOSITE SECTION PROPERTIES	(PROPERTIES CALCULATED FROM BUILT-UP SECTION)									
	AREA	12	ZY	AZ2	120	ZY	ITOT			
	(IN2)	(IN2)	(IN2)	(IN2)	(IN2)	(IN2)	(IN2)			
CEDER ONLY	558.0	25.7	14364.	464175.	92241.	186659.				
SLAB ONLY	775.2	60.2	46667.	2609356.	3731.					
FILLET ONLY	45.6	25.2	2517.	138945.	22.					
COMB ONLY	.0	.0	0.	0.	0.	0.				
COMP SEC PROP	1378.8	46.1	61548.	3412476.	9594.	379549.				
CALCULATED SIMPLY SUPPORTED SPAN MOMENTS										
SM0 = DEAD LOAD OF CEDER ONLY	=	465.1								
SM0 = DEAD LOAD OF SLAB + FILLET	=	684.2								
SM0 = ADDED DEAD LOAD	=	195.0								
SM0 = LIVE LOAD MOMENT	=	889.0								
SMUAR = ULTIMATE MOMENT APPLIED	=	2671.1								
PERIODIC CEDER ANALYSIS										
GROSS AND RESISTANCE - GROSS CROSS-SECTIONS										
	MOMENT (IN')	ZT (IN2)								
SL CEDER	465.	845.								
SL SLAB + FILLET	684.	1242.								
ADDED SL	195.	32.								
TOTAL SL	1344.	2120.								
(LL+I)S	888.	145.								
TOTAL SL = (LL+I)	—	3265.								
DESIGN STRESS - F DUE (PSI) (ONE OF THE FOLLOWING)										
	F1 = 6 * SQRT(F'0') * ALL. CON. FACTOR	=	2004.							
	F2 (LL + ADD. DG)	=	2088.							
FIRM TRANSFORMED CEDER PROPERTIES										
	(PROPERTIES CALCULATED FROM BUILT-UP SECTION)									
	AREA	12	ZY	AZ2	120	ZY	ITOT			
	(IN2)	(IN2)	(IN2)	(IN2)	(IN2)	(IN2)	(IN2)			
GROSS GER SEC	558.0	25.7	14364.	464175.	92241.	186659.				
(LL+I)AS	20.2	6.0	121.	729.	—					
TOTAL GER SEC	578.2	25.3	14485.	464304.	92241.	194274.				



0 FINAL TRANSFORMED COMPOSITE GIRDER PROPERTIES (PROPERTIES CALCULATED FROM BUILD-UP SECTION)					
		(I _{ZZ} = I _{ZZ} + A _Z * Z _Z)		I _{ZZ}	
INCH	IN	A _Z	A _Z	Z _Z	I _{ZZ}
SPAN COMP GDR	1386.0	45.5	63570.	3413205.	96564.
					611638.
					$I_{ZZ}^2/I_{ZZ} = 1.6413$
					$I_{ZZ}^2/I_{ZZ} = 1.4209$

STRUCTURE W/O PRESTRESS - USING FINAL TRANSFORMED CROSS-SECTIONS					
MOMENT (K')	FT (KSI)	IN (KSI)			
SL. GIRDERS	465.	832.	-720.		
SL. SLAB + FELLET	684.	1223.	-1059.		
ADDED DL.	185.	32.	-174.		
TOTAL DL.	1344.	2083.	-1953.		
DL+TDS	888.	149.	-730.		
TOTAL DL. + (DL+TDS)	—	2235.	-2743.		

DESIGN STRESSES - P DRS (PSI) (ENTER ONE OF THE FOLLOWING)					
P _D = 6 * SQRT(G'') * ALL. TENS. FACTOR =	2366.	PS (DL + ADD. DL) =	1953.		
PRESTRESSED CIRCUIT ANALYSIS					

PRESTRESSING FORCE - PRESTRESSED					
ALLOW. TENSION = (0.8 * .75) = 35.00 = 167500. (PSI)					FINAL PRESTRESSED FORCE = P _T = 563200. (PSI)
AREA OF PRESTRESSING STEEL - A _{ST} = 3.37 (IN ²)					INITIAL PRESTRESSED FORCE = P _T = 639470. (PSI)
STRUCTURE WITH PRESTRESS - USING FINAL TRANSFORMED CROSS-SECTIONS (PSI)					

AT MIDSPAN TOP FIBER BOTTOM FIBER					
6 SPANNING (PSI)	122.	671.	1957.		
6 SL. GIRDERS (PSI)	1428.	6787.	588.		
6 FULL DES. LOAD (PSI)	1609.	6787.	-375.		

CONCRETE STRENGTH REQUIRED (PSI)					
P _C = P _{MAX} /1.60 = 3339.	P _C = P _{MAX} /1.40 = 4620.	PS _{MAX} (PSI) = 4620.			
DEFLECTIONS (INCHES) COMMAND POSITION					

DP1= -1.6724	DP2= .7632	DP3= -.0052	DP4= -1.0879	DP5= 1.1227	DP6= .1016	DP7= .1364
CHECK ULTIMATE RESISTING MOMENT						

MOMENT, INDEX = .045						
P _C (PSI) = 263603.						
ULTIMATE RESISTING MOMENT (U-RM) = 3971.09						
N.A. & ULTIMATE LOAD (IN) = 3.71						

PRESTRESSED CIRCUIT ANALYSIS					
PROFILE - FINAL PRESTRESSING FORCE CALCULATED					

NO.	TOTAL STRENGTH	COMPOSITE	FINAL STRENGTH	FINAL FORCE	ULTIMATE MOMENT	REFLECTIONS (SEE HMC)	SECTION PROPERTIES											
							TOP FIBER	TOP BOTTOM	BOT. FIBER	TOP. FIBER	AVL'D	RIGID	SL.	DP1	DP2	DP3	DP4	DP5
0 1	122. 1957. 1609. -375.	3.29 4.02	565.	3671.1	3971.1	-1.139	-0.076	.694	.611	134273	578	25.1	611638	45.5				

NOTE: DEFLECTIONS ARE POSITIVE DOWNWARD

NOTE: DEFLECTIONS ARE BASED ON P-S CONCRETE

NOTE: FOR P-S CONCRETE P_C IS 4320. PSI



PRESTRESSED GIRDER ANALYSIS													
INPUT DATA									PAGE 5				
T E		P											
R E		R		L									
I G		S		O									
A T		T E											
L H		T E											
A T		X Y	X	X	I	FINAL	LIVE	TO					
L H			C	M	R	ALL	FREIGHT	TO					
A T			C	M	R	ALL	FREIGHT	INITIAL					
L H			C	M	R	ALL	FREIGHT	FINAL					
(IN)	(IN)	(IN)	(IN)	(IN)	(IN)	(IN)	(IN)	(IN)					
(IN)	(IN)	(IN)	(IN)	(IN)	(IN)	(IN)	(IN)	(IN)					
2.0	64.0	54.0	102.4	14.0	80.	7.6	162.	1.1	270.				
NOTE: ASSUMED DATA IS BRACKETTED BY COLONS.													
ANALYSIS	TRAIL NO. 2	PRE-TENSIONED											
GROSS SECTION PROPERTIES - GIRDERS		(PROPERTIES CALCULATED FROM BUILT-UP SECTION)											
		(ITOT = 100 + AY2 - SUM (AY * YR))											
	AREA	YR	AY	AY2	100	ITOT							
	(INCH)	(INCH)	(INCH)	(INCH)	(INCH)	(INCH)							
	GROSS GSR SEC	558.0	25.7	14344.	464175.	92241.	186639.						
GROSS COMPOSITE SECTION PROPERTIES		(PROPERTIES CALCULATED FROM BUILT-UP SECTION)											
		(ITOT = 100 + AY2 - SUM (AY * YR))											
	AREA	YR	AY	AY2	100	ITOT							
	GIRDER ONLY	558.0	25.7	14344.	464175.	92241.	186639.						
	SLAB ONLY	775.2	69.2	44667.	2909356.	3731.							
	PILELET ONLY	45.6	55.2	2517.	128945.	22.							
	COVERING ONLY	,0	,0	,0	,0	,0							
	COMP SEC PROP	1378.8	46.1	63248.	3412476.	96994.	1879109.						
CALCULATED SIMPLY SUPPORTED BEAM MOMENTS													
	MDL = DEAD LOAD OF GIRDERS ONLY	=	460.4										
	MDL = DEAD LOAD OF SLAB + PILELET	=	684.2										
	MDL = ADDED DEAD LOAD	=	105.0	IMPUTED VALUE									
	MLL4 = LIVE LOAD MOMENT	=	971.0	IMPUTED VALUE									
	MLLL4 = ULTIMATE MOMENT APPLIED	=	3839.9	1.3*(MDL+MLL4)*MLL4	+ 2.17*MLL4								
PRESTRESSED GIRDER ANALYSIS													
STRENGTH W/O PRETENSION - USING GROSS CROSS-SECTIONS													
	MOMENT (IN')												
	DL GIRDERS		460.	845.		-770.							
	DL SLAB + PILELET		684.	1241.		-1132.							
	ADDED DL		105.	32.		-186.							
	TOTAL DL		1344.	2120.		-2088.							
	(LL+I)R		971.	159.		-927.							
	TOTAL DL + (LL+I)			2279.		-3015.							
DESIGN STRESS = F TENS (PSI) (CREATEA OF THE FOLLOWING)													
		FD = 4 * 302(G' C) * ALL. TENS. FACTOR = 2631.											
				FD (DL + ADD. DL) = 2088.									
	FINAL TRANSFORMED GIRDER PROPERTIES		(PROPERTIES CALCULATED FROM BUILT-UP SECTION)										
		(ITOT = 100 + AY2 - SUM (AY * YR))											
	AREA	YR	AY	AY2	100	ITOT							
	(INCH)	(INCH)	(INCH)	(INCH)	(INCH)	(INCH)							
	GROSS GSR SEC	558.0	25.7	14344.	464175.	92241.	186639.						
	(H-1)*AS'	22.9	6.0	137.	624.	---							
	TRANS GSR SEC	580.9	25.9	14501.	464999.	92241.	186228.						
	TRANSFORMED COMPOSITE GIRDERS PROPERTIES		(PROPERTIES CALCULATED FROM BUILT-UP SECTION)										
		(ITOT = 100 + AY2 - SUM (AY * YR))											
	AREA	YR	AY	AY2	100	ITOT							
	(INCH)	(INCH)	(INCH)	(INCH)	(INCH)	(INCH)							
	TRANS GSR SEC	1411.7	45.4	63685.	3413303.	96994.	1815755.						
		(G' * YR) / R**2 = 1.6384											
		(E' * YR) / R**2 = 1.4056											



STRENGTHS W/O PRESTRESS - USING FINAL TRANSFORMED CROSS-SECTIONS

	MOMENT (IN')	PT (PSI)	PB (PSI)
SL CENTER	465.	630.	-714.
SL SLAB + FILLET	684.	1221.	-1050.
ADDED DL	195.	33.	-173.
TOTAL DL	1344.	2064.	-1936.
ULL+DL	971.	162.	-860.
TOTAL DL + ULL+DL	---	2246.	-2796.

PRESTRESSED GIRDERS ANALYSIS

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PRESTRESSING FORCE - PRESTRESSED

$$\text{ALLOW. TENSION} = [(P_S + .75) \times 35.00] = 167500. \text{ (PSI)}$$

AREA OF PRESTRESSING STEEL - ACT = 3.81 (IN²)

FINAL PRESTRESS FORCE - IV = 630000. (LBS)

INITIAL PRESTRESS FORCE - VI = 722228. (LBS)

STRENGTH WITH PRESTRESS - USING FINAL TRANSFORMED CROSS-SECTIONS (PSI)

AT MIDSPAN	TOP FIBER	BOTTOM FIBER
1. PRESTRESS (PSI)	36.	(PSI)
2. SL CENTER+SLAB (PSI)	1349.	(PSI)
3. FULL DES. LOAD (PSI)	1544.	(PSI)

CHECK ULTIMATE RESISTING MOMENT

$$P_U = F_{MAX}/.60 = 3357. \quad P_C = 33600/.40 = 3350. \quad P_{MAX} (\text{PSI}) = 4000.$$

DEFLECTIONS (INCHES) (DOMINANT POSITIVE)

$$\Delta F_1 = -.0739 \quad \Delta g = .7614 \quad \Delta w = -1.1145 \quad \Delta \theta = -1.3623 \quad \Delta L_1 = 1.1200 \quad \Delta \alpha_{12} = .1012 \quad \Delta \theta = -.1411$$

CHECK ULTIMATE RESISTING MOMENT

$$\text{REDNF. INDEX} = .052 \quad \text{ULT. RESIST. MOMENT (K-FT)} = 4458.88$$

PBU (PSI) = 242768. N.A. & ULTIMATE LOAD (IN) = 4.19

PRESTRESSED GIRDERS ANALYSIS

PAGE 8

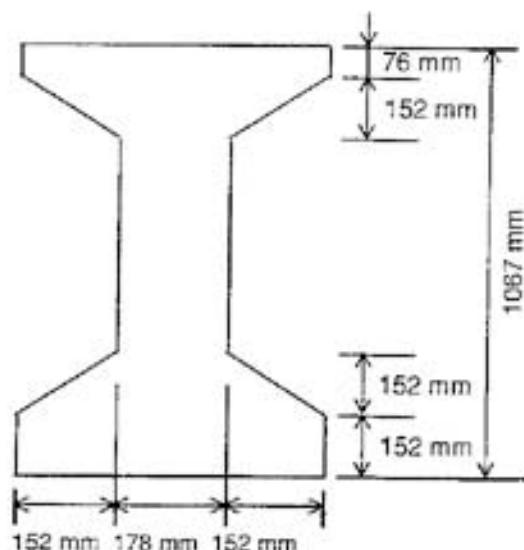
RESULTS - FINAL PRESTRESSING FORCE GIVEN

TRAIL INPUT	CONCRETE	ULTRASO	DEFLECTIONS (SEE HMC)	FINAL STRESSES			SECTION PROPERTIES			
				NO. PREST. STRANDS	MOMENT	DEFL	DEFL	DEFL	TOP	BOTT.
FORCE INCH. FINAL	APPLD	PSI	INCH.	INCH.	INCH.	(PSI)	(PSI)	(INCH.)	(PSI)	(INCH.)
(KIPS) (PSI) (K-FT)	(K-FT)	(K-FT)	(INCH.)	(INCH.)	(INCH.)	(PSI)	(PSI)	(INCH.)	(PSI)	(INCH.)
0 2 639. 3.94 3.96 3350.9 4458.8 -.156 -.093 .053 -.012 1.1200	1. PRESTRESS	36.	2284.	581.	196228.	25.0				
	SL SLAB	1349.	886.	581.	196228.	25.0				
	ADD. D.L.	1382.	713.	1402.	615755.	45.4				
	FULL DES. LOAD	1544.	-147.	1402.	615755.	45.4				

NOTE: DEFLECTIONS ARE POSITIVE DOMINANT

NOTE: DEFLECTIONS ARE BASED ON TRAIL P-S P/C

NOTE: FOR P-S CONCRETE P/C USE 4000. PSI

Example Problem 3 (Metric)
Standard Symmetric "T" Girder

Using section dimension cards

Girder spacing = 2235 mm
 Span length = 20.7 m
 Slab thickness = 180 mm
 Structure depth = 1235 mm

STATE OF CALIFORNIA • DEPARTMENT OF TRANSPORTATION CALTRANS DESIGN SYSTEM PRESTRESSED GIRDER ANALYSIS (CS-D-001B REV. 11/97)															A Metric						
Trial No	Girder Properties			Composite Slab						Flexibility Data				Moments				Line Load Length			
	Section Depth (mm)	Girder Depth (mm)	Girder Span Length (m)	HSL	BSL	Length	Thickness	Width	Slab	Tc	Flexibility Coefficient C	XCL	XL0	Flexibility Coefficient C	Flexibility Factor F	Liner Load Length	Line Load Length	Line Load Length	Line Load Length	Line Load Length	Line Load Length
1	1520	1520	20.7	1520	1520	20.7	152	152	180	1235	0.1	1520	1520	0.1	1	1520	1520	1520	1520	1520	1520
1	1520	1520	20.7	1520	1520	20.7	152	152	180	1235	0.1	1520	1520	0.1	1	1520	1520	1520	1520	1520	1520
1	1520	1520	20.7	1520	1520	20.7	152	152	180	1235	0.1	1520	1520	0.1	1	1520	1520	1520	1520	1520	1520

STATE OF CALIFORNIA • DEPARTMENT OF TRANSPORTATION CALTRANS DESIGN SYSTEM PRESTRESSED GIRDER ANALYSIS - SECTION DIMENSIONS (CS-D-002B REV. 11/97)															D Metric		
Trial No	HXT	HXB	H4L	H5L	H6L	H7L	H8L	H9L	H10L	B5L	W	B4L	B5L	B6L	B7L	B8L	S
	(mm)	(mm)															
1	1520	1520	178	152	152	152	152	152	152	152	152	152	152	152	152	1520	
1	1520	1520	178	152	152	152	152	152	152	152	152	152	152	152	152	1520	
1	1520	1520	178	152	152	152	152	152	152	152	152	152	152	152	152	1520	



Output for Example Problem 3:

CALIFORNIA DEPARTMENT OF TRANSPORTATION
PRESTRESSED CONCRETE ANALYSIS
(VERSION 2.00)

PAGE 1

TRAIL SECTION DIMENSIONS

TRAIL NO.	SYMMETRIC SECTION											
	BLT	EDB	HAL									
1	0	0	76	152	152	152	0	0	152	176	152	0

PRESTRESSED CONCRETE ANALYSIS

PAGE 2

INPUT DATA

	P	R	S	T	W	DAM	DAM	DAM	APPLIED	APPLIED	APPLIED	APPLIED
T S	X	X	X	X	L	FLOOR	LIVE	TO	TO	END	TO	TO
R E		C	N	A	ALL	FRONT	LOAD	INITIAL	INIT.	FLOOR	FLOOR	FLOOR
I C				L	0	X TIN	FORCE	SECTION	SECTION	SECTION	SECTION	SECTION
A T	STRUCTURE	CROSS	P-S	SLAB	--	COMPOSITE	SLAB	--	P-S	C	N	
L W	DEPTH	DEPTH	SPAC	P-C	LONG	THICK	WEIGHT	SPAC	P-C	EIN	F'S	L
	(MM)	(MM)	(MM)	(MM)	(MM)	(MM)	(MM)	(MM)	(MM)	(MM)	(MM)	(MM)
1.0	1273	1067	2235	28.0	20.7	180	2235	.0	22.5	0.1	1862	89

NOTE: ASSUMED DATA IS INDICATED BY COLONS.

ANALYSIS TRAIL NO. 1 PRE-CONDITIONED

GROSS SECTION PROPERTIES - CIRCLE (PROPERTIES CALCULATED FROM BUILT-UP SECTION)

	AREA	YD	AY	AX2	Z00	ZTOT
(10^3mm^2)	(MM)	(10^3mm^2)	(10^6mm^4)	(10^6mm^4)	(10^6mm^4)	(10^6mm^4)
GROSS GSR AREA	305	508	155019	100166	18178	39669

GROSS COMPOSITE SECTION PROPERTIES (PROPERTIES CALCULATED FROM BUILT-UP SECTION)

	AREA	YD	AY	AX2	Z00	ZTOT
CIRCLE ONLY	305	508	155019	100166	18178	39669
SLAB ONLY	402	1183	475921	563014	10466	
FILLET ONLY	13	1060	13535	14617	1	
CIRCLE ONLY	0	0	0	0	0	
COMP SEC FRCF	720	895	644474	677797	122463	122414

CALCULATED SIMPLY SUPPORTED BEAM MOMENTS

MOM = DEAD LOAD OF CIRCLE ONLY =	385.7
MOM = DEAD LOAD OF SLAB + FILLET =	523.8
MOM = ADDED DEAD LOAD =	296.3 INPUTTED VALUE
MOM = LIVE LOAD MOMENT =	1679.4 8220-44 CHECK LOADING
MOM = ULTIMATE MOMENT APPLIED =	3621.7 $1.3 \times (MOM + MHD \times 0.06) + 1.3 \times (P \times TOT)$

PRESTRESSED CONCRETE ANALYSIS

PAGE 3

STRENGTH W/O STRENGTH - USING GROSS CROSS-SECTIONS

	MOMENT (KN-m)	PT (GPa)	FH (GPa)
DL. CIRCLE	385.	5439.	-4534.
DL. SLAB + FILLET	524.	7387.	-6701.
ADDED DL	296.	427.	-2217.
 TOTAL DL	 1206.	 13263.	 -13851.
(LL+I)X	1579.	1544.	-8020.
 TOTAL DL + (LL+I)	 ---	 14797.	 -21871.

DESIGN STRESSES - F DEX (GPa) (GREATER OF THE FOLLOWING)

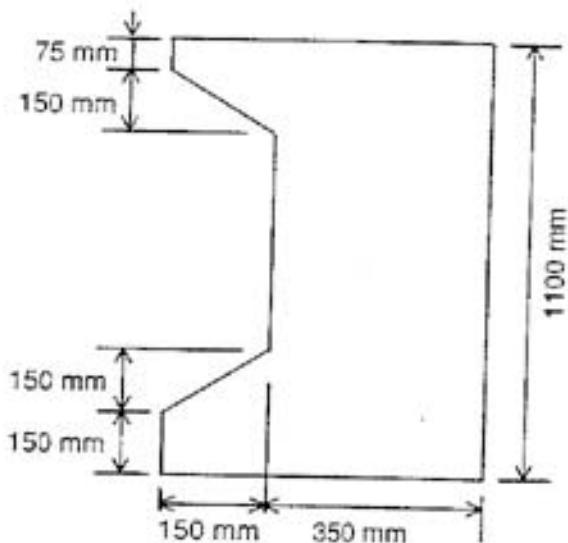
FH = 0.54*FCM(F'c) * ALL TENS. FACTOR =	19218.	FH (DL + ADD. DL) =	13851.
NOTE :- REAL CALCULATIONS ARE IN ENGLISH UNITS			



FINAL TRANSFORMED GIRDERS PROPERTIES		PROPERTIES CALCULATED FROM SHELL-OF-SECTIONS					
		AREA	YB	AX	AX2	I00	ITOT
(10'3" x 10')	880	(10'3" x 10')	(10'3" x 10')	(10'3" x 10')	(10'3" x 10')	(10'3" x 10')	(10'3" x 10')
GROSS GIRD. SEC.	305	258	155010	100366	18178	20669	
SH-11 "AS"	12	29	1020	92	—	—	
TRAN. GIRD. SEC.	317	432	156047	100257	18138	41613	
FINAL TRANSFORMED COMPOSITE GIRDERS PROPERTIES		PROPERTIES CALCULATED FROM SHELL-OF-SECTIONS					
		AREA	YB	AX	AX2	I00	ITOT
TRAN. COMP. GIRD.	741	882	645500	677000	19565	127797	
SPUTT./R**2 = 1.763				SPUTT./R**2 = 1.5120			
STRENGTH W/O PRESTRESS - USING FINAL TRANSFORMED CROSS-SECTIONS							
		MOMENT (KN-m)		PT (kPa)		TB (kPa)	
DL. GIRDER	386			5325		-4513	
DL. SCAR + FILLET	524			7223		-6195	
ARMED SL	295			412		-2059	
TOTAL DL	1204			12060		-12813	
(LL+I)S	1079			1242		-7443	
TOTAL SL + (LL+I)S	—			24582		-20264	
DESIGN STRESS - F DLS (kPa)	(CREATE OF THE FOLLOWING)						
Fs = 0.590710 * ALL. TENS. FACTOR = 17000.				FS. GL + ADD. SL		= 12915.	
NOTE :- ALL CALCULATIONS ARE IN ENGLISH UNITS							
PRESTRESSED GIRDER ANALYSIS							
PRESTRESSING FORCE - PRESTRESSED							
ALLOW. TENSION = (Fy) = .75 = 241.320 = 3154813. (kN)							
AREA OF PRESTRESSING STEEL - ASt = 1.92 (10^3 mm^2)							
STRENGTH WITH PRESTRESS - USING FINAL TRANSFORMED CROSS-SECTIONS (kPa)							
AT MIDSPAN		TOP FIBER		BOTTOM FIBER			
0 STRESSING (PTF)	-746	(PTF)	15509				
0 DL. GIRDOR+SL (PTF)	7135	(PTF)	6840				
0 FULL DLS. LOAD (PTF)	9285	(PTF)	-2647				
AT 1/3 POINT ALONG SPAN LENGTH	MOMENTS SHARED AT 1/3 POINTS						
0 STRESSING (PTF)	-1333	(PTF)	15871				
0 FULL DLS. LOAD (PTF)	5818	(PTF)	8044				
0 FULL DLS. LOAD (PTF)	7584	(PTF)	-415				
CHECKED STRENGTH REQUIRED (kPa)							
PTC = PMAX/.40 = 26442		PC = PMAX/.40 = 22360		FORCE (kN)		= 27994	
DEPLACEMENTS (mm)	(DOMINANT POSITION)						
DY= -44.37	DX= 16.51	DD= -29.04	DPY= -34.74	DDX= 22.43	DPD= 4.16	DPY= -8.15	
CHECK ULTIMATE RESISTING MOMENT							
RESCD. DDX = .058				UL. RESCD. MOMENT (KN-m) = 3772.91			
PCD. 0581 = 1805627.				U.A. @ ULTIMATE LOAD (kN) = 35.66			
RESULTS - FINAL PRESTRESSING FORCE CALCULATED							
INITIAL STRESSES	CONCRETE	FINAL	ULTIMATE	DEPLACEMENTS	SECTION PROPERTIES		
NO.	INITIAL	FINAL	DESCRIPT.	FORCE	AREA	YB	SL
FOR BOTTOM TOP BOTTOM UNIT. FINAL FORCE	APPLIED	RETU	PTF	DESF1 DESF2 DESF3	1	2	3
(kPa) (kPa) (kPa) (kPa) (kPa)	(kPa)	(kPa)	(kPa)	(kPa)	(kPa)	(kPa)	(kPa)
6 1 -746 15509 9135 -2647 26.4 23.0 2223	3521	3773	-44.6 -29.1 22.4 -8.1 41613	317 432 127797 882			
NOTE: DEPLACEMENTS ARE POSITIVE DOMINANT							
NOTE: DEPLACEMENTS ARE BASED ON INPUT P-S F'C							
NOTE: FOR P-S CONCRETE F'C USE 27994. kPa							
NOTE: APPLIED ULTIMATE MOMENT EXCEEDS ULTIMATE RESISTING MOMENT. HOLD STEEL IS REQUIRED.							

Example Problem 4 (Metric)

Non-Standard Non-Symmetric "T" Girder



Using section dimension cards

Girder spacing = 2300 mm

Span length = 20.5 m

Slab thickness = 200 mm

Trial No.	Girder Properties										Prestress Data						Moments				
	Sectionals		Dimensions		Properties		Concrete Slab		Prestressing		Loadings		Live Load		Note A		Note B		Note C		
	Bottom Depth (mm)	Side Depth (mm)	Shear Span (m)	P.S. Span Length (m)	Thickness (mm)	Width (mm)	Slab (mm)	Tc (mm)	Bottom Distance (mm)	Fy (kN/mm)	XCL (mm)	ZND (mm)	Live Load Type	Prestress Force (kN)	Lanes	DLM Applied to Initial Section (kNm)	DLM Applied to Final Section (kNm)	DLM Applied to TPD Initial Section (kNm)	DLM Applied to TPD Final Section (kNm)	LL+IM Applied to Final Section (kNm)	
1	1100.00	150.00	15.00	17.50	20.0	1200	200	150	150	400	1100.00	1100.00	150	400	1	1	1	1	1	12.93	1

Trial No.	STATE OF CALIFORNIA • DEPARTMENT OF TRANSPORTATION CALTRANS DESIGN SYSTEM PRESTRESSED GIRDER ANALYSIS - SECTION DIMENSIONS DS-D-0025 (REV. 11/97)															Metric	
	Symmetrical Sections																
HXT	HXB	H4L	H5L	H6L	HTL	H8L	H9L	B5L	W	B4L	B5L	B6L	B7L	B8L	S		
(mm)	(mm)	(mm)	(mm)	(mm)	(mm)	(mm)	(mm)	(mm)	(mm)	(mm)	(mm)	(mm)	(mm)	(mm)	(mm)		
1	1100	150	150	175	1150	1150	1150	150	150	1150	1200	1150	1150	1150	1	A	
1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	B	

Trial No.	STATE OF CALIFORNIA • DEPARTMENT OF TRANSPORTATION CALTRANS DESIGN SYSTEM PRESTRESSED GIRDER ANALYSIS - SECTION DIMENSIONS DS-D-0025 (REV. 11/97)												Metric	
	Unsymmetrical Sections													
B3R	B4R	B5R	B6R	B7R	B8R	H4R	H5R	H6R	H7R	H8R	H9R			
(mm)	(mm)	(mm)	(mm)	(mm)	(mm)	(mm)	(mm)	(mm)	(mm)	(mm)	(mm)	(mm)	(mm)	
1	1150	150	150	150	150	1150	1150	1150	150	150	150	150	150	C
1	1	1	1	1	1	1	1	1	1	1	1	1	1	D



Text File for Example Problem 4

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'3 This panel program is written by Ray Singh.'  
'5 Output is in metric Units.'  
'5 Input is in Metric units.'  
'5 Output, Input, Free or Fixed Input in English'  
'Units' 'M' 'M' 'F'  
  
'3A Trg, Sec#, Strc D, Gr, f'c, SpLn, Sl, T, W, Supt, f'c, Ty, Prc, f's, NCL, XND, Ldw, All F, MPP, DLL, DLM1, DLM2, DLM3, DLM4'  
'X' 1 0 1300.0 1100.0 2300 .0 20.5 200.0 2300 .0 .0 1 1 0 100.0 500.0 1 1.0 0 .0 0 0 200 0  
  
'SD Trg, EXT, HCR, HCL, HSL, HYL, HPL, HRL, HBL, W, B4L, BSL, BCL, B7L, BSL, S'  
'D' 1 .0 .0 75.0 150.0 150.0 .0 .0 150.0 200.0 150.0 .0 150.0 1 150.0 0  
  
'SE Trg, B2R, B4R, B5R, B6R, B7R, B8R, H4R, H5R, H6R, H7R, H8R, H9R'  
'E' 1 150.0 .0 .0 .0 150.0 1100.0 .0 .0 .0 .0 .0 .0
```

Output for Example Problem 4:

CALIFORNIA DEPARTMENT OF TRANSPORTATION												PAGE 1			
PRESTRESSED GIRDER ANALYSIS															
(VERSION 2.00)															
TRAIL SECTION DIMENSIONS															
TRAIL COMPOSITE SECTION															
NO.	RECT	HCR	HCL	HSL	HYL	HPL	HRL	HBL	W	B4L	BCL	B7L	B8L	S	
1	0	0	75	150	150	150	0	0	150	201	150	0	150	0	150
TRAIL UNCOMPOSITE SECTION															
NO.	B2R	B4R	B5R	B6R	B7R	B8R	B9R	B10R	B11R	B12R	B13R	B14R	B15R	B16R	
1	150	0	0	0	0	150	1100	0	0	0	0	0	0	0	
PRESTRESSED GIRDER ANALYSIS														PAGE 2	
INPUT DATA															
T	P	R	L	E	O										
T,S	T														
R,E	T,E														
I,C	Y,N	X	X	L	FINAL	LIVE	TO	D	ED	ID	ID				
A,T	SIMU,GDR,GDR,P-S	GIR	GIR		C	N	A,LL	BLDG	LOAD	INITIAL	BLDG	FINAL			
L,N	DEPTH,DEPTH,SEC	F'C	IING	THICK,WITH,SUR,F'C,EN,F'S	L	D	X,TEN	FORCE,LINES	SECTION,SEC.	SEC.	SEC.				
mm.)	(mm.)	(mm.)	(mm.)	(mm.)	(mm.)	(mm.)	(mm.)	(mm.)	(mm.)	(mm.)	(mm.)	(mm.)	(mm.)	(mm.)	
1.0	1300	1100	2300	28.0	20.5	200	2300	.0	22.5	1.1	1862	100	500	Y1.0	
NOTE: ASSUMED DATA IS ENCODED BY COLUMNS.															